



UNIVERSITY OF SÃO PAULO
SÃO CARLOS SCHOOL OF ENGINEERING
DEPARTMENT OF STRUCTURAL ENGINEERING



THE ERGODICITY ASSUMPTION IN PERFORMANCE-BASED ENGINEERING

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Dr. Rubia M. Bose, M. Isabela D. Rodrigues



Motivation

Study of Time-variant Reliability.

Motivation

Time-variant Reliability is **dead**!

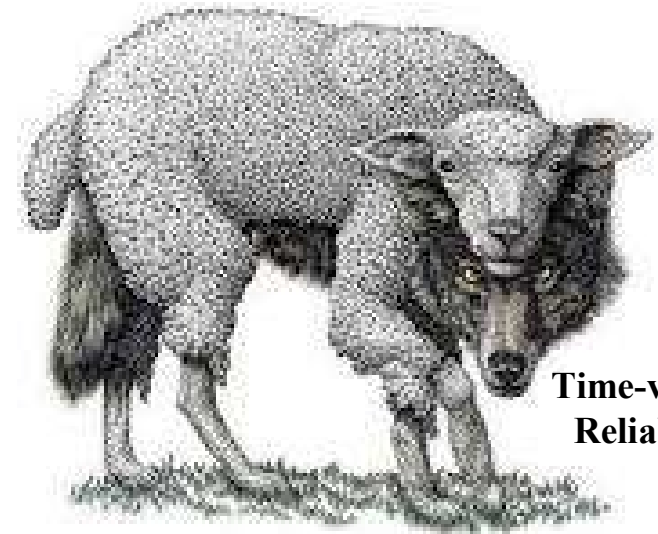
Motivation

Time-variant Reliability analysis is **dead!**

It has been updated to:

Performance Based Engineering.

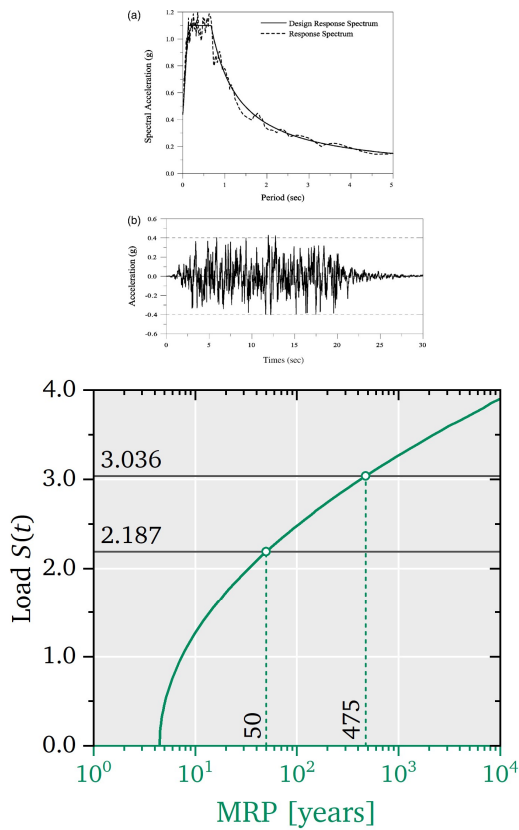
Performance-Based Engineering



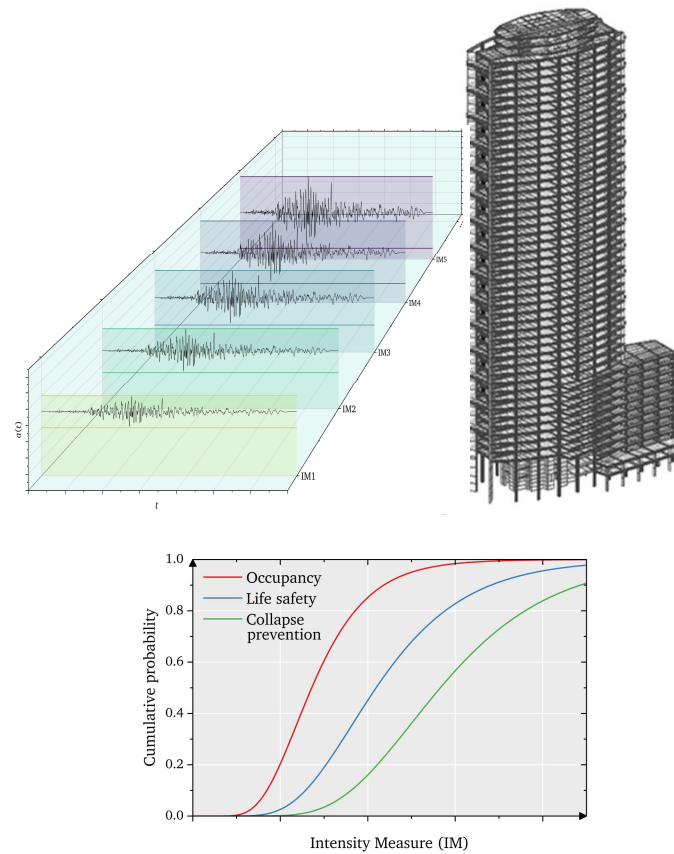
Time-variant
Reliability

Performance Based Engineering

Hazard analysis (IM):

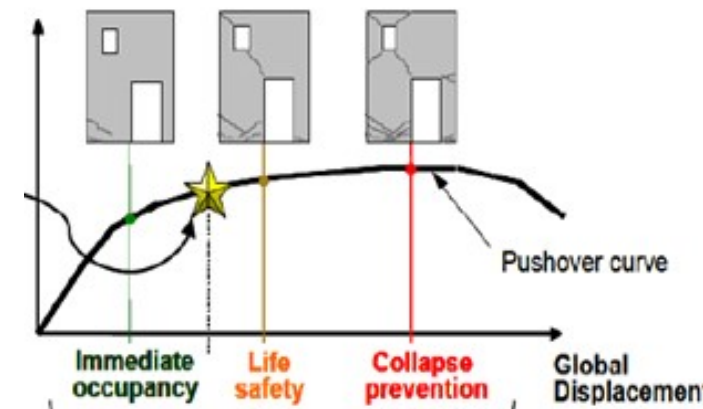


Demand analysis (EDP):



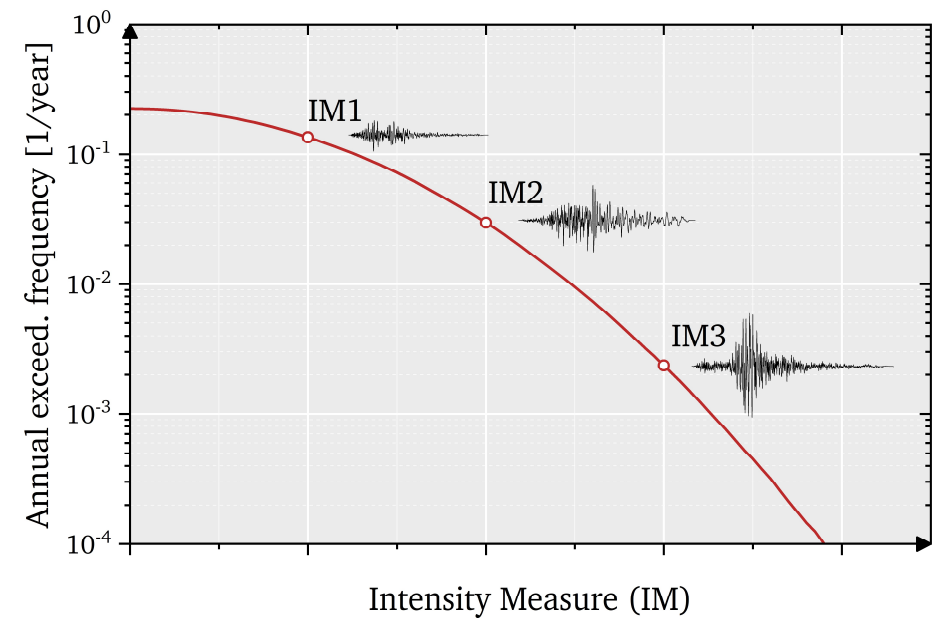
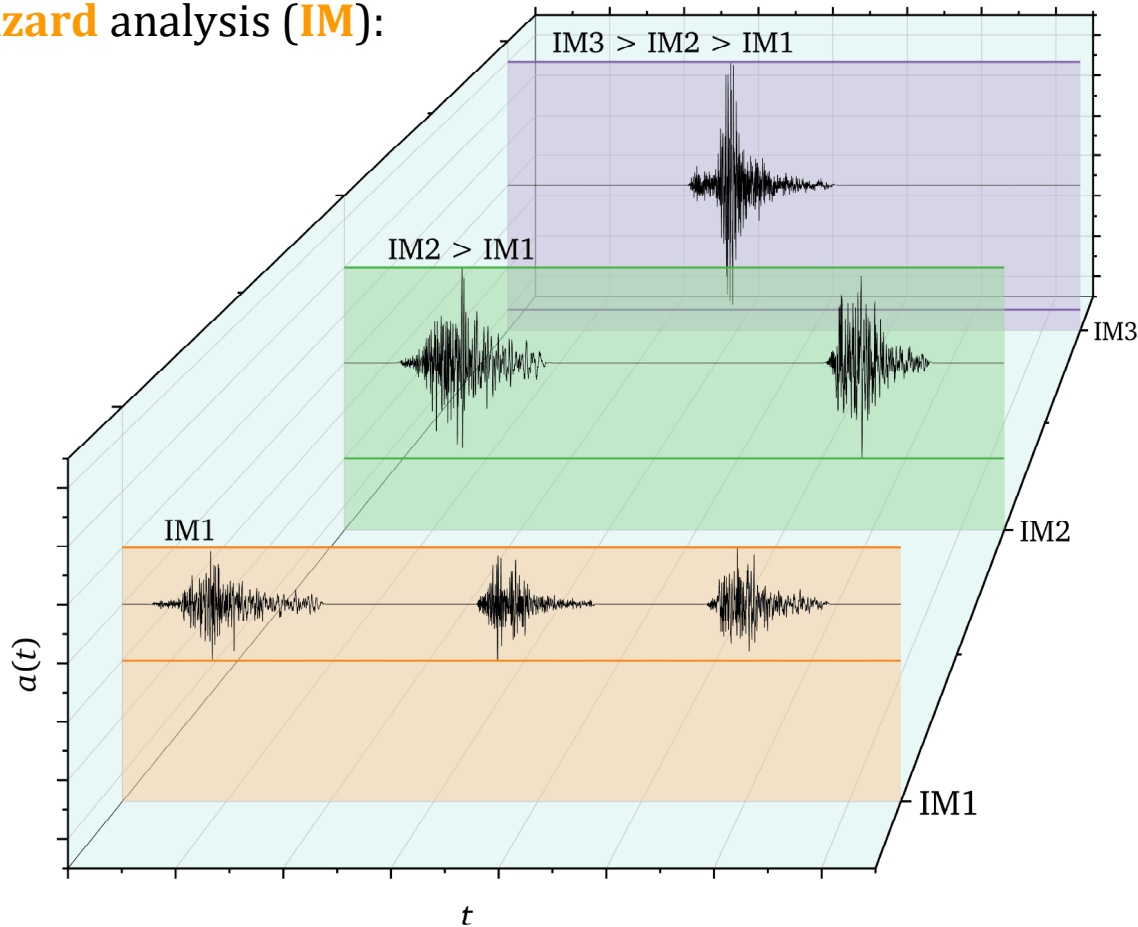
Damage & loss analysis (DM):

Static pushover analysis:



Performance Based Engineering

Hazard analysis (IM):



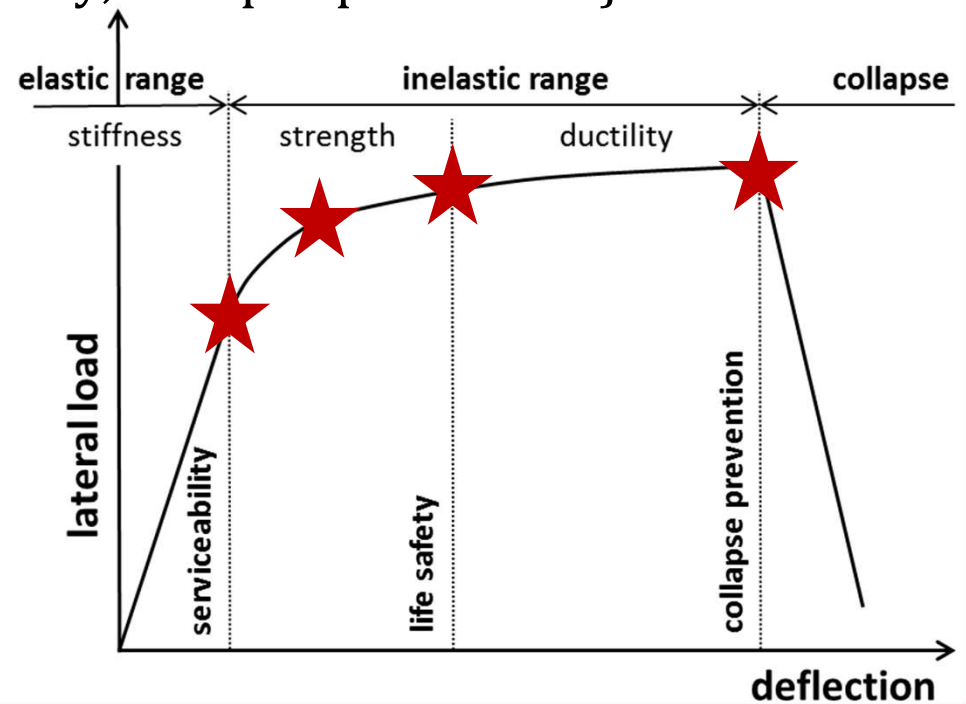
Performance Based Engineering

Demand analysis (**EDP**):

Limit states:

$LS(\mathbf{EDP}) = \{\text{Operational, damage control, life safety, collapse prevention}\}$

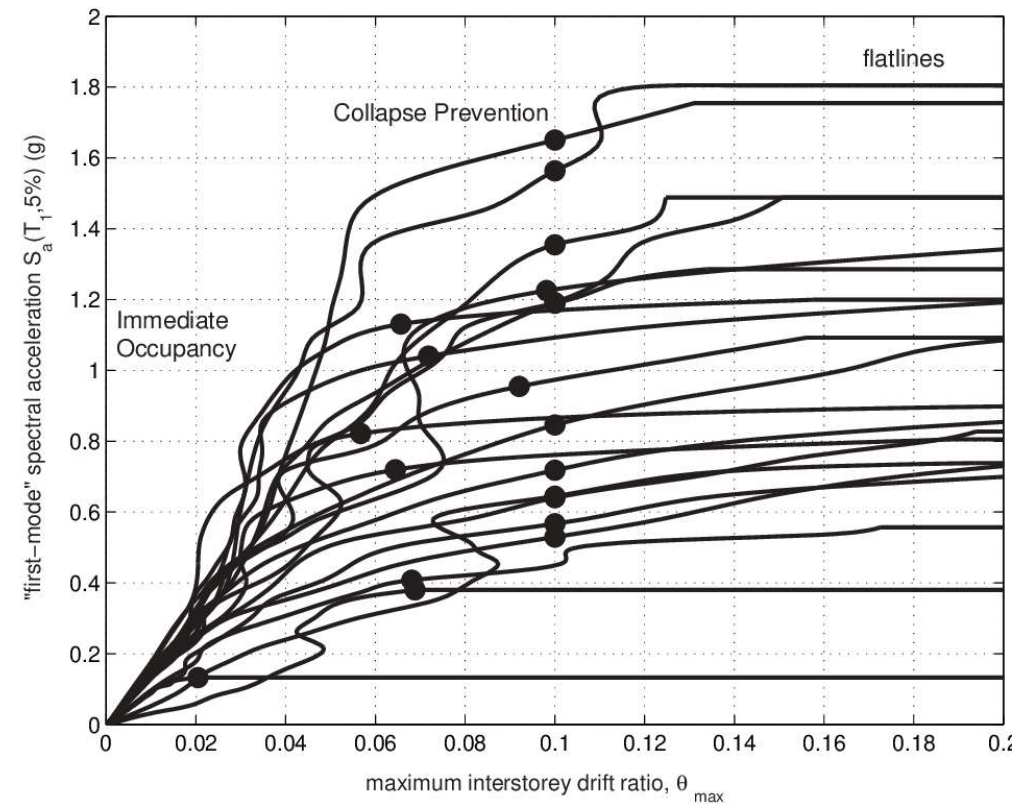
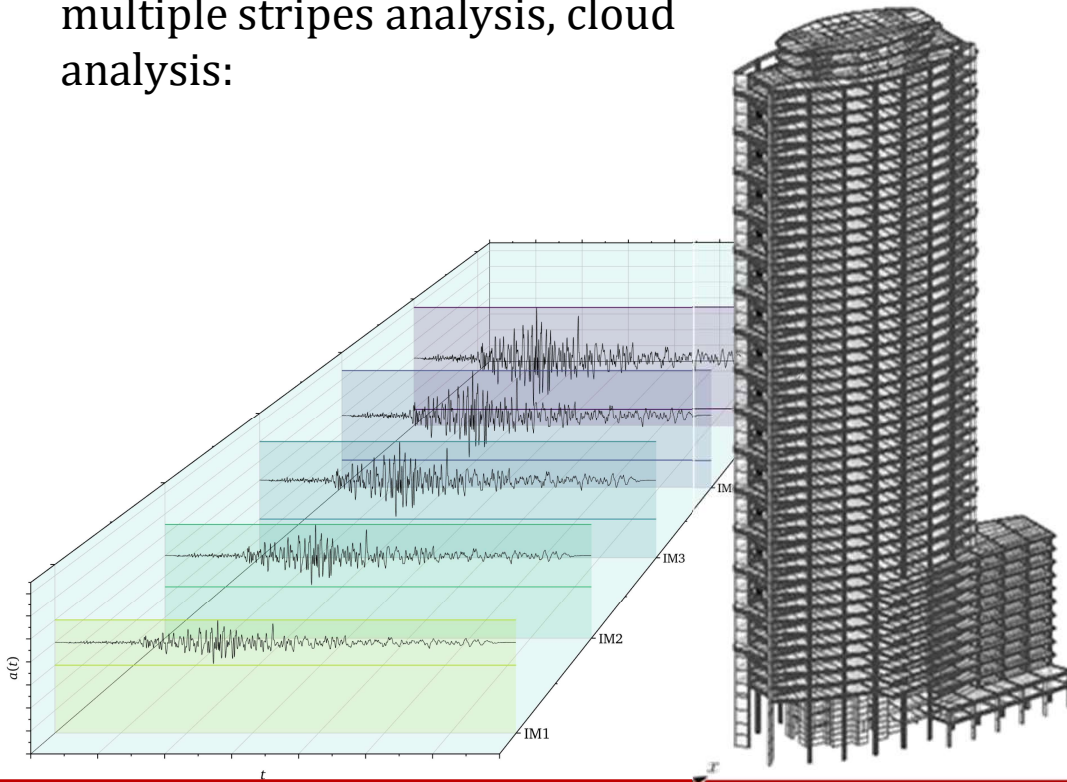
EDP = *Engineering demand parameter*
Displacements;
accelerations;
interstory drift ratios, etc.



Performance Based Engineering

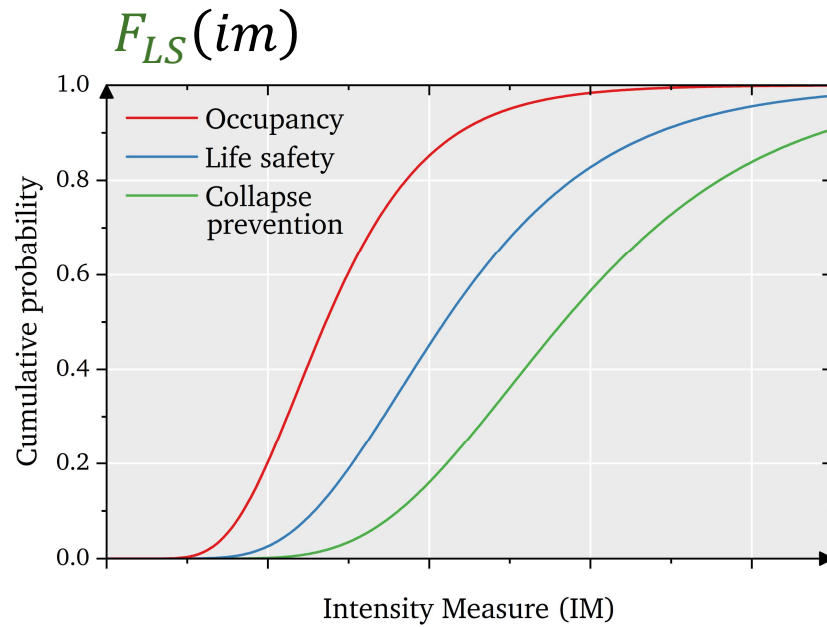
Demand analysis (**EDP**):

Incremental dynamic analysis (IDA),
multiple stripes analysis, cloud
analysis:



Performance Based Engineering

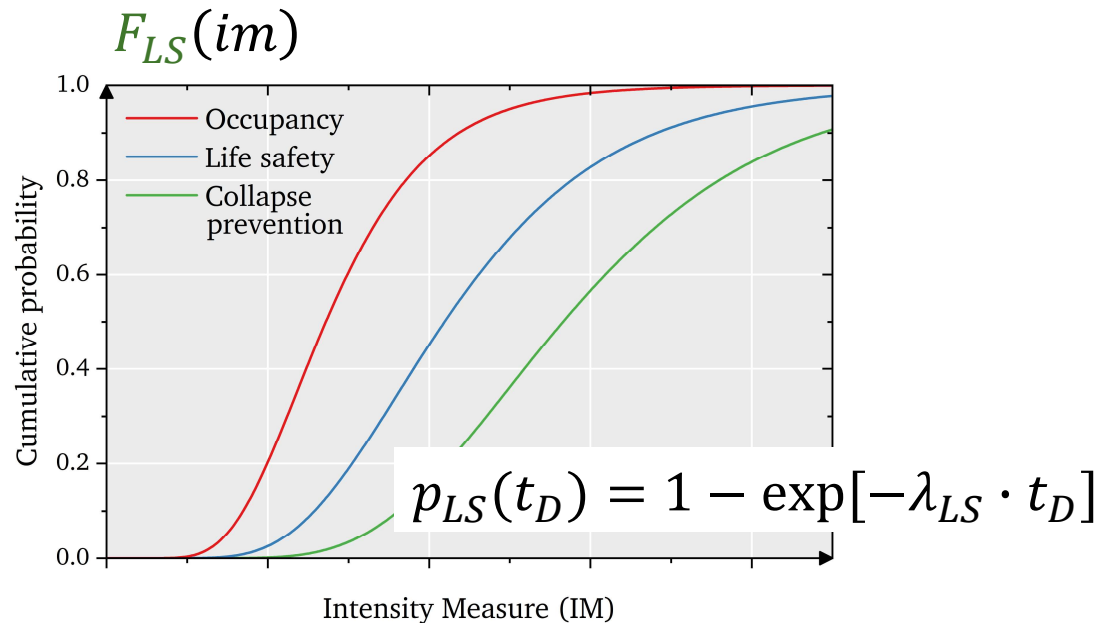
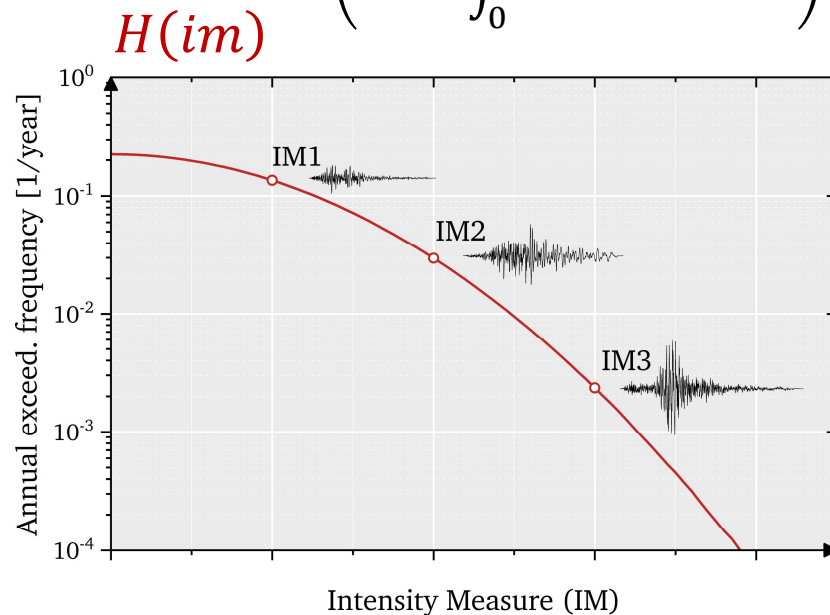
Fragility curves: $F_{LS}(im) = P[g_{LS}(\mathbf{x}|IM = im) \leq 0]$
 $= \int \int F(LS|\mathbf{EDP})f(\mathbf{EDP}|\mathbf{SP}, IM) f(\mathbf{SP}, IM) d\mathbf{EDP} d\mathbf{SP}$



Performance Based Engineering

Fragility curves: $F_{LS}(im) = P[g_{LS}(\mathbf{x}|IM = im) \leq 0]$
 $= \int \int F(LS|\mathbf{EDP}) f(\mathbf{EDP}|\mathbf{SP}, IM) f(\mathbf{SP}, IM) d\mathbf{EDP} d\mathbf{SP}$

Annual failure rate: $\lambda_{LS} = \int_0^\infty |h(im)| F_{LS}(im) dim = \int_0^\infty f_{LS}(im) H(im) dim$ $h(im) = \frac{dH(im)}{dim}$
 $\left(p_f = \int_0^\infty f_S(s) F_R(s) ds \right)$



Time-variant Reliability

$$p_f(\mathbf{r}, t_D) = p_{f0} + (1 - p_{f0}) \left(1 - \exp \left[- \int_0^{t_D} \eta(\mathbf{r}, t) dt \right] \right) \\ \geq 1 - \exp \left[- \int_0^{t_D} \eta(\mathbf{r}, t) dt \right]$$

$\eta(\mathbf{r}, t)$ Rate of a non-homogeneous
Poisson process;
 \mathbf{r} Vector of system parameters;
 $E_{\mathbf{R}}[.]$ Expectation over \mathbf{R} .

$$p_f(t_D) = E_{\mathbf{R}}[p_f(\mathbf{r}, t_D)] = \int_{\mathbf{R}} \left(1 - \exp \left[- \int_0^{t_D} \eta(\mathbf{r}, t) dt \right] \right) f_{\mathbf{R}}(\mathbf{r}) d\mathbf{r}$$

Time-variant Reliability in PBE

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$$p_f(t_D) = E_{\mathbf{R}} \left[1 - \exp \left(- \int_0^{t_D} E_{\mathbf{RTR}}[\eta(\mathbf{r}, im, t)] dt \right) \right]$$

$E_{\mathbf{RTR}}[.]$ Expectation over variables with **Record-to-Record** variability: intensity, time-history, spectral shape;

$$p_f(t_D) = \int_{\mathbf{R}} \left[1 - \exp \left(- \int_0^{t_D} \int_{IM} \lambda(\mathbf{r}, im, t) h(im) dim dt \right) \right] f_{\mathbf{R}}(\mathbf{r}) d\mathbf{r}$$

Ergodic variables: value changes from one load application to the next!
Non-ergodic variables: value is the same for all load applications!

Time-variant Reliability in PBE

Integration over im leads to the annual failure rate:

$$\lambda(\mathbf{r}, t) = \int_{IM} \lambda(\mathbf{r}, im, t) h(im) dim$$

$$p_f(t_D) = \int_{\mathbf{R}} \left[1 - \exp \left(- \int_0^{t_D} \lambda(\mathbf{r}, t) dt \right) \right] f_{\mathbf{R}}(\mathbf{r}) d\mathbf{r}$$

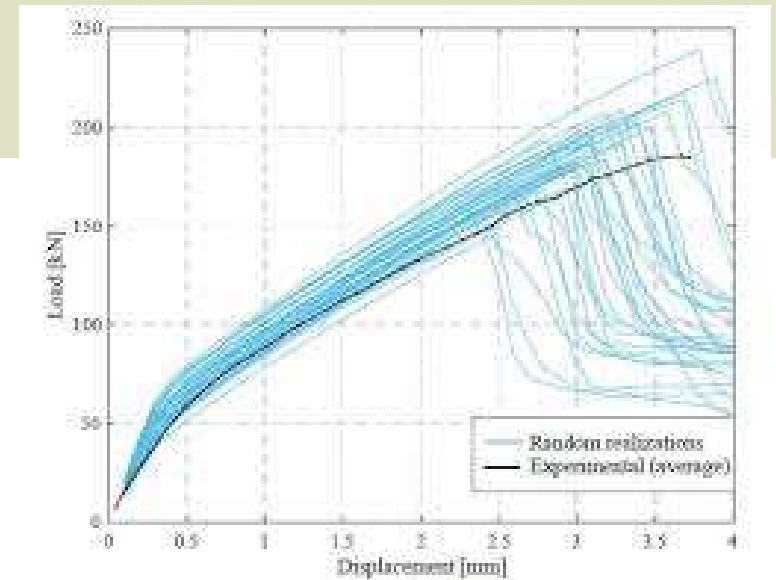
Each sample of random system parameters should be exposed to the whole suit of ground motions: $N \times N_m$ non-linear structural dynamics time-history computations;

N is the number of ground motion records;

N_m is the number of system parameter samples.

Instead:

$$\lambda_E(t) = \int_{IM} \int_{\mathbf{R}} \lambda(\mathbf{r}, im, t) f_{\mathbf{R}}(\mathbf{r}) d\mathbf{r} h(im) dim$$



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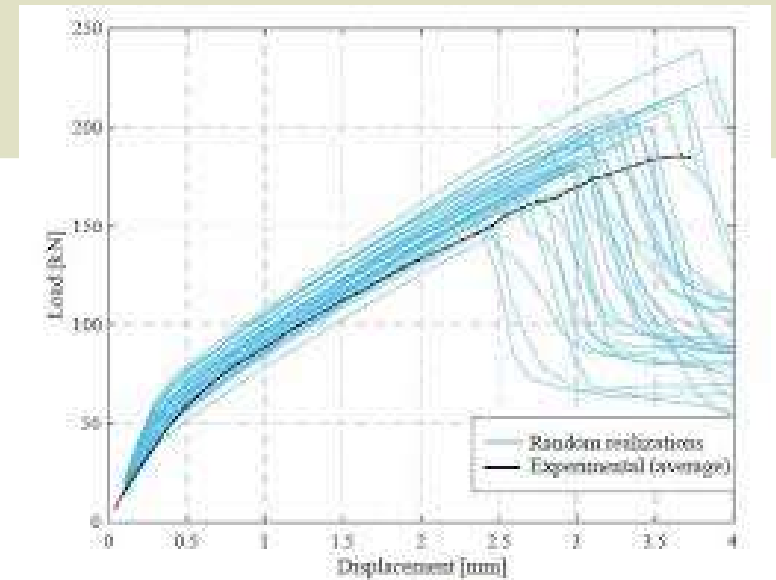
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**Offends the Poisson assumption
of independent crossings!**

Der Kiureghian, 2005: Non-ergodicity and PEER's framework formula, *Earthq. Eng. Struct. Dyn.* 34.
Jalayer & Ebrahimi, 2020: Seismic reliability assessment and the nonergodicity in the modelling parameter uncertainties," *Earthq. Eng. Struct. Dyn.* 49.



Fragility function as a product of lognormals

$$\lambda_{LS} = \int_0^{\infty} F_Z(IM_C | im) h(im) dim$$

$$Z = g(X, Y) = X \cdot Y$$

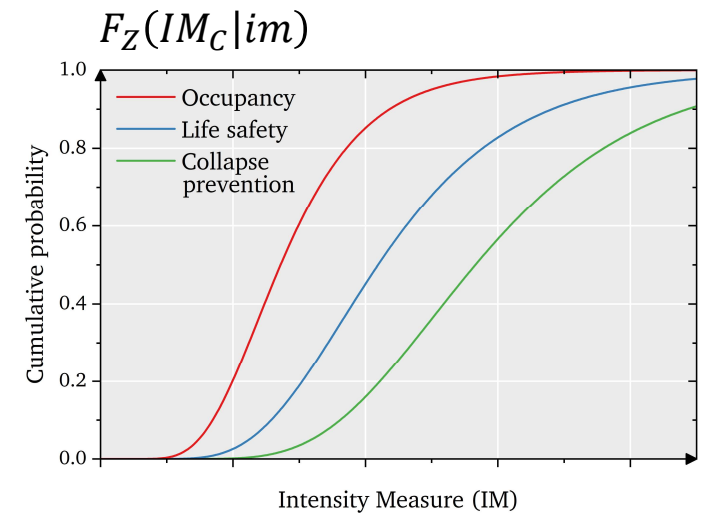
Beck et al. 2022: On the ergodicity assumption in Performance-Based engineering, Structural Safety 97.

X effect of all ergodic variables, but the hazard intensity
(RTR variability)

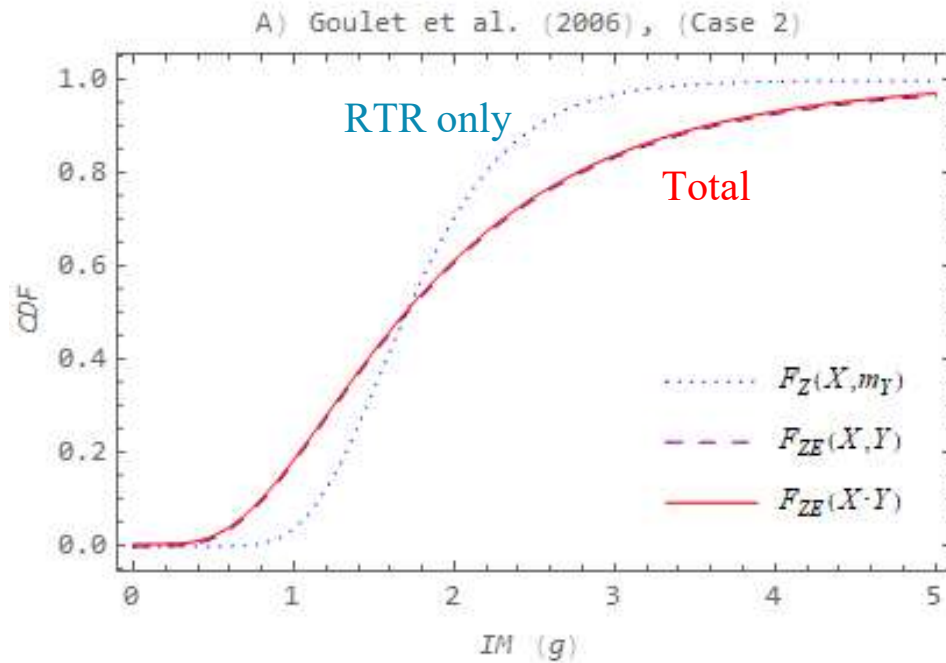
Y is the effect of random system parameters: $Y = w(\mathbf{R})$.

$$E[\ln(Z)] = E[\ln(X)] + E[\ln(Y)] = \hat{m}_X + \hat{m}_Y$$

$$Var[\ln(Z)] = Var[\ln(X)] + Var[\ln(Y)] = \sigma_{\ln X}^2 + \sigma_{\ln Y}^2$$

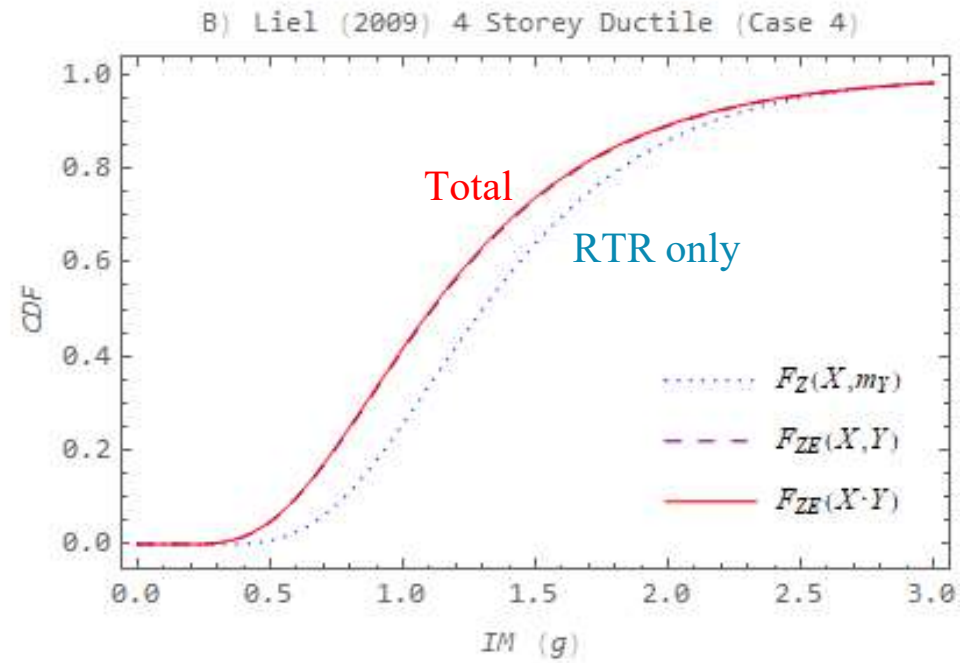


Realistic fragility curves



4-story frame from:

Goulet et al. 2006: Evaluation of the seismic performance of a code-conforming reinforced-concrete frame building - Part I: Ground motion selection and structural collapse simulation.



4-story frame from:

Liel et al. 2009: Incorporating modeling uncertainties in the assessment of seismic collapse risk of buildings, Structural Safety 31.

Evaluation of the “ensemble crossing rate” error:

$$Z = g(X, Y) = X \cdot Y \quad x = g^{-1}(z, y) = z/y$$
$$\partial g^{-1} / \partial z = 1/y$$

$$F_Z(z) = \int_0^\infty \int_0^{g^{-1}} f_{XY}(x, y) dx dy = \int_0^\infty \int_0^z \frac{1}{y} f_{XY}\left(\frac{z}{y}, y\right) dz dy$$

$$F_Z(z) = \int_0^\infty \int_0^z \frac{1}{y} f_X\left(\frac{z}{y}\right) f_Y(y) dz dy$$

Beck & Melchers, 2004: On the ensemble crossing rate approach to time variant reliability analysis of uncertain structures, *Probabilistic Eng. Mech.* 19.

Beck & Melchers, 2005: Barrier failure dominance in time variant reliability analysis, *Prob. Eng. Mech.* 20.

Beck, 2008: The random barrier-crossing problem, *Probabilistic Eng. Mech.*, vol. 23

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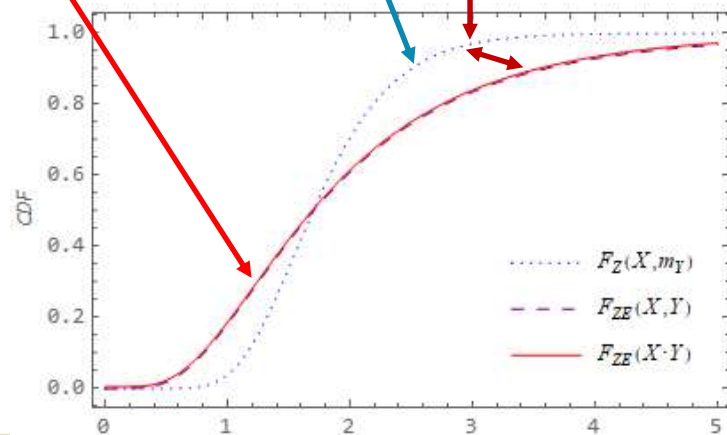
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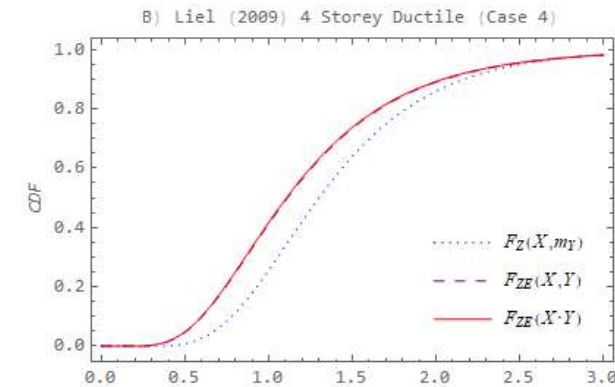
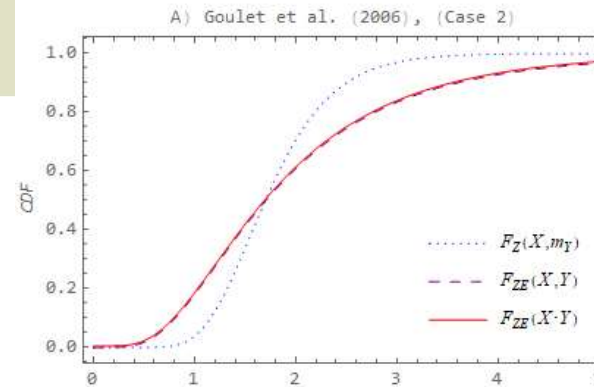


$$Z(y) = X \cdot y \quad \text{Record-to-Record variability only!}$$

$$\lambda_{LS}(y) = \int_0^\infty F_X(IM_C | im, y) h(im) dim$$

$$p_f(t_D) = \int_0^\infty \left[1 - \exp\left(-\int_0^{t_D} \lambda_{LS}(y) dt\right) \right] f_y(y) dy$$

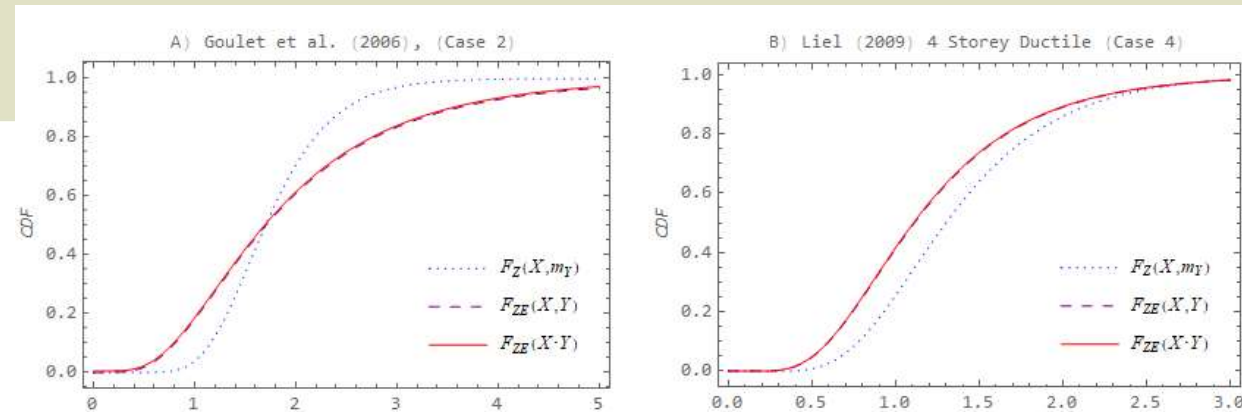
Realistic fragility curves:



Ref.	Case	Stories	T_1 (s)	Margin w.r.t. $S_a(T_1)_{2/50}$	RTR (X)		Total (Z)		System only (Y)	
					\hat{m}	σ_{ln}	\hat{m}	σ_{ln}	\hat{m}	σ_{ln}
[8]	1	4	1.00	2.11	1.7	0.30	1.7	0.58	1	0.5
	2		1.00	2.61	2.1	0.29	2.1	0.578	1	0.5
	3		1.00	3.48	2.8	0.34	2.8	0.605	1	0.5
[9]	4	4	1.12	1.52	1.3	0.40	1.10	0.48	0.85	0.26533
	5	12	2.01	1.23	0.61	0.473	0.56	0.52	0.918	0.21603
	6	12	1.98	0.63	0.3	0.45	0.28	0.50	0.933	0.21795
	7	12	2.26	0.73	0.35	0.415	0.38	0.49	1.086	0.26053

Error evaluation

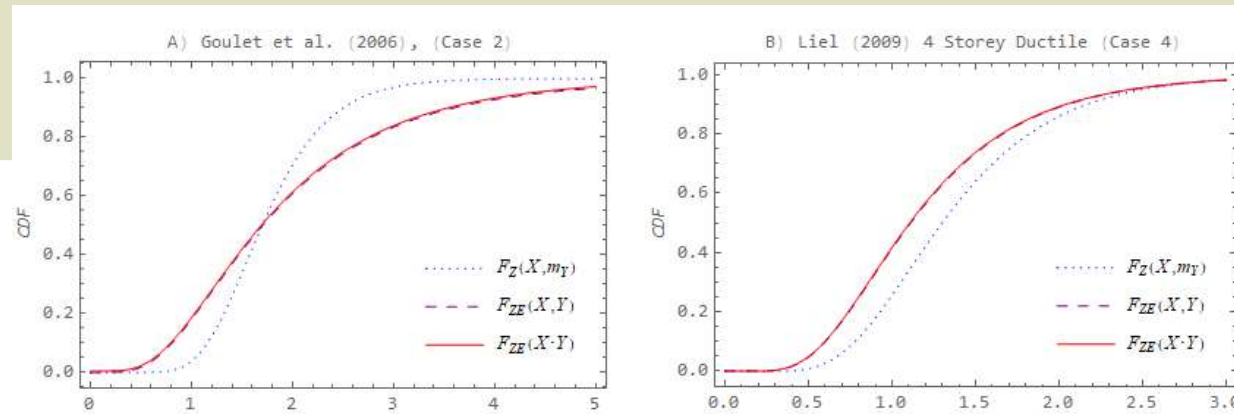
Annual collapse rates $\lambda_{LS} \times 10^4$:



Frame	a) RTR only	b) Total, (exact)	c) Total, with ER error	Ratio b/a (effect of system uncertainty)	% Error $\lambda 100(c-b)/b$
1	0.215	2.564	2.569	11.92	0.17
2	0.049	1.087	1.088	22.20	0.11
3	0.013	0.422	0.422	31.55	0.07
4	2.413	6.743	6.746	2.80	0.04
5	6.790	11.086	11.089	1.63	0.03
6	51.168	67.903	67.969	1.33	0.10
7	30.937	30.565	30.592	0.99	0.09

Error evaluation

Fifty-year $p_f(t_D = 50) \times 10^4$:



Frame	a) RTR only	b) Total, (exact)	c) Total, with ER error	Ratio b/a (effect of system uncertainty)	% Error $\lambda 100(c-b)/b$
1	0.0011	0.0119	0.0128	11.03	7.62
2	0.0002	0.0052	0.0054	21.05	5.27
3	0.0001	0.0020	0.0021	30.60	3.05
4	0.0120	0.0326	0.0332	2.72	1.79
5	0.0334	0.0532	0.0539	1.59	1.40
6	0.2257	0.2772	0.2881	1.23	3.94
7	0.1433	0.1363	0.1418	0.95	4.07

Concluding remarks

Offending the **Poisson** assumption of independent crossings can be avoided in PBE by a *product-of-lognormals* scheme;



Results are particularly relevant for **service limit states** in *Performance Based Wind Engineering*, since the average number of load cycles is much larger (to be investigated);

Details in Beck et al. 2022: On the ergodicity assumption in Performance-Based engineering, Structural Safety 97.

Acknowledgements:

Thanks to:



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The colleagues for your attention.

Contact: Prof. André T. Beck, atbeck@sc.usp.br

